STEEP WAVE AND BREAKING WAVE IMPACT ON OFFSHORE WIND TURBINE FOUNDATIONS - RINGING RE-VISITED

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1. INTRODUCTION

In these experiments we report on the interaction of steep waves, both non-breaking and breaking, hitting a bottom-founded vertical circular cylinder. Both the scattered wave surface elevations around the column and the total horizontal forces were measured. In contrast to most previous experiments we concentrate on hitting the column with localised wave groups not with regular or random waves. By repeating the tests with both focussed wave groups and the same wave groups inverted, so each tall crest in one is replaced by a deep trough in the inverted form, we are able to identify unambiguously the complete Stokes expansion type representation of the applied force: linear, second order two-frequency sum and difference, third order triple-frequency sum etc. The cumulative effect of this relatively small but significant high frequency excitation is to produce dynamic response or ‘ringing’ of the column if the lowest resonant frequency is in the right range. Remarkably, much of this harmonic structure survives even with violently breaking waves.

Offshore wind turbine farms are constructed in areas of high winds where large waves will also occur. Thus, wave induced loads are an important design constraint. In this work we concentrate on nonlinear components of the hydrodynamic loading on a surface piercing column as a model of the base of an offshore wind turbine. The loads which we identify are potential in form – drag forces appear to be negligible, consistent with a Keulegan-Carpenter parameter \( KC\sim O(10) \) for the largest waves reported here.

Most wave-structure interaction experiments are performed either with regular waves or in realisations of random sea-states. The first lacks any representation of the broadband spectrum of real ocean waves, the second suffers problems of wave reflections in finite sized tanks and also the rarity of extreme wave events of interest for design in a random sea-state. In contrast, we have performed a large set of focussed wave group tests, where both the frequency spectrum and phase of the components are carefully controlled. In these tests, the key feature is the use of wave groups. The aim of this is twofold: first to ensure that each short test contains an interaction of interest, and second to allow the unambiguous extraction of the harmonic structure of the fluid loading on a vertical column.

2. EXPERIMENTS

The shallow water basin at DHI was used for these tests. A vertical cylinder of diameter 0.25m was suspended from a stiff triangular frame via a load-cell. For the tests reported here the water depth across the basin was constant at 0.505m and the cylinder extended downwards to the basin floor, leaving a thin gap of 1mm beneath. The cylinder was located at 7.8m from the paddles in the centre of the tank. An array of wave gauges was used to monitor the wave-field around the cylinder, pressure gauges were installed at 4 vertical locations on the front stagnation line of the cylinder and the wave kinematics were measured with an ADV. A range of wave conditions was tested, from small close to linear waves up to spilling and plunging breakers. In each case a compact wave group focussed at the
front stagnation point of the cylinder was used to avoid reflections from the basin walls. Fig. 1 shows an incident breaking wave (left) and the consequences of such a wave hitting the column (right).

FIGURE 1. Left: Breaking wave about to hit the vertical cylinder, showing the wave gauge array and the cylinder supported from above. Right: Vertical sheet of water wrapped around the front of the cylinder during a wave impact.

In other experiments, the cylinder was embedded in a caisson on the bed of the basin, and also located midway up a 1:20 plane beach on a small horizontal ledge at a depth of 0.5m. Further data analysis is underway to compare the effects of these changes in wind turbine foundation geometry and also the nature of the incident waves for the sloping bed case on the force time history.

Figure 2 shows two force time-histories for wave groups on constant depth hitting the cylinder. The spectrum of the NewWave-type focussed wave group is JONSWAP ($\gamma=3.3$) in shape with a peak frequency of 0.61Hz. The upper part of the figure shows the free-surface time history in the absence of the cylinder and the horizontal force on the cylinder, both resulting from a relative small non-breaking unidirectional wave group (steepness $AK=0.197$ and depth $KD=0.995$, both non-dimensionalised with the wavenumber corresponding to the spectral peak frequency). The middle section of the figure shows the incident free-surface motion and the force in the same now mean-wave direction caused by a bi-directional wave system consisting of two unidirectional wave groups crossing at ±20°. Each of these component groups is the same as that producing the force record in the upper part of the figure. The two groups cross at the cylinder, producing a combined breaking wave of twice the height of the individual groups in isolation. Whilst wave height is locally doubled, the peak horizontal force is greater than 3× and there is clear evidence of ‘ringing’ force components on the column both before and after the instant when this peak force occurs.

Both the incident wave free-surface elevation in the absence of the cylinder and the horizontal force time-histories contain significant higher harmonic structure well above the linear frequency range, as can be seen at the bottom of Figures 2. The resonant frequency of the ‘wet’ cylinder on its load-cell support was 3.8Hz, so force harmonics up to the 5th are not significantly affected by the system dynamics. The data sample rates used were at least 100Hz in all tests. From both of the bottom subplots of Figure 2, the possibility of the excitation of the ‘ringing’ response of cylinder at its natural frequency is clear. This high frequency excitation of a surface piercing column beyond the 2nd harmonic of the incident waves, albeit here in a shallow water wind turbine context, is comparable to the excitation of deep water concrete structures of concern to the oil industry in the 1990s, see Faltinsen et al. (1995), Newman (1996), Chaplin et al. (1997). However, we observe not only the large 3rd order component discussed in the FNV-model but also 4th and 5th harmonic excitation as well.

The various harmonics can be extracted by combining the time-histories for the force for a crest-focussed group (C, say) with that for exactly the same wave packet but inverted (T), obtained by multiplying the paddle command signal by -1. These are combined into (C-T)/2 which contains linear, 3rd order, 5th order etc., and (C+T)/2 containing 2nd order difference, 2nd order sum, 4th order… The individual components can then be extracted by digital filtering. This technique assumes the existence of a Stokes-like harmonic series in both frequency and wave steepness. It has been successfully applied to surface elevation data in a study of wave scattering off the bow of a grossly
simplified FPSO geometry by Zang et al. (2006); and previously to shoaling waves on a plane beach by Hunt et al. (2002), Borthwick et al. (2006).

The analysis of Zang et al. (2006), based on model tests at Imperial College, showed large 2nd order scattering from a cylindrical front of a long box with head-on waves but no significant higher harmonics beyond 2nd. In direct contrast, the surface elevation spectra and particularly the forces measured on the circular cylinder in this work show that all harmonics up to at least the 6th are important. So, there is a sense in which the rear half of the cylinder is required for there to be large components beyond the 2nd order sum harmonic.

In order to identify the order and time-scale of the various harmonic components, we also show wave envelopes for each harmonic component on Figure 3. The estimated envelope of each harmonic is derived from the envelope of the linear force (3rd plot down). The square of the linear envelope is scaled to fit the size of the maximum of the measured 2nd harmonic, the cube of the linear envelope to the 3rd etc. This localisation in time of the higher harmonics works well for all terms up to the 5th harmonic, showing the value of the idea of a generalised Stokes-type expansion for a wave group. The terms beyond 5th (on the figure the 5th power of the linear envelope is plotted again) contain the ‘ringing’ mechanical response of the column, all the force harmonics below this excite the column in a quasi-static manner.

In this context, the second pulse of waves in the second harmonic component, in both cases centred at +6s, is an error wave train generated by the paddle which takes longer to reach the model than the main group. These error waves are observed at the location of the cylinder at this time even when the cylinder was removed. They arise because only linear transfer functions are used to drive the paddles, but the waves which are created are inherently nonlinear so instantaneous cancellation occurs between the correct 2nd order bound waves for the main group and these error components which then propagate down the tank as free components.

Analysis of the harmonic structure of the applied force is still underway. At the workshop we hope to report on the comparison of linear and 2nd order diffraction theory to the first 2 harmonic components and the FNV model to the 3rd.
3. CONCLUSION

A single vertical cylinder was exposed to a large series of carefully controlled incident wave groups. Manipulation of the phase of these groups allows extraction of the harmonic structure of the loading. Even for violently breaking waves, much of the harmonic structure of the resulting horizontal loading is still apparent and consistent with that measured for smaller non-breaking waves.

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REFERENCES


