

# Towards development of a nonlinear perturbation method for analysis of springing of ships

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## 1. Introduction

A time domain method is employed to analyze interactions between waves and floating bodies. The nonlinear free surface conditions and body boundary conditions are satisfied based on the perturbation method up to 3<sup>rd</sup> order. The objective is to theoretically study springing of ships. Springing is a weakly nonlinear problem. The relevant wave lengths are short relative to the ship length. Thus, for the ship springing problem, from computational efficiency point of view, a perturbation method has its advantage compared with a fully nonlinear method.

## 2. Description of the method

The 1<sup>st</sup>, 2<sup>nd</sup> and 3<sup>rd</sup> order free surface conditions follow by introducing the perturbation expansions of velocity potential  $\phi = \phi_1 + \phi_2 + \phi_3 + O(\varepsilon^4)$  and wave elevation  $\eta = \eta_1 + \eta_2 + \eta_3 + O(\varepsilon^4)$ :

$$\begin{cases} \eta_t^{(m)} = \phi_z^{(m)} - F_1^{(m)}, & \text{on } z=0 \\ \phi_t^{(1)} = -g \cdot \eta^{(1)} + F_2^{(1)}, & \text{on } z=0, \quad m=1,2,3 \end{cases}$$

Where  $F_1^{(1)} = 0$ ;  $F_2^{(1)} = 0$ ;

$$F_1^{(2)} = -\phi_x^{(1)} \eta_x^{(1)} - \eta^{(1)} \phi_{xx}^{(1)};$$

$$F_2^{(2)} = -\frac{1}{2} |\nabla \phi^{(1)}|^2 - \eta^{(1)} \phi_{zt}^{(1)}$$

$$F_1^{(3)} = -\phi_x^{(1)} \eta_x^{(2)} - \phi_x^{(2)} \eta_x^{(1)} - \eta^{(1)} \phi_{xx}^{(2)} - \eta^{(1)} \eta_x^{(1)} \phi_{zx}^{(1)} - \eta^{(2)} \phi_{xx}^{(1)} - 0.5(\eta^{(1)})^2 \phi_{zxx}^{(1)}$$

$$F_2^{(3)} = -\nabla \phi^{(1)} \cdot \nabla \phi^{(2)} - \eta^{(1)} \phi_{zt}^{(2)} - \eta^{(1)} \nabla \phi^{(1)} \cdot \nabla \phi_z^{(1)} - \eta^{(2)} \phi_{zt}^{(1)} + 0.5(\eta^{(1)})^2 \phi_{lxx}^{(1)}$$

The small parameter  $\varepsilon$  here is a measure of the wave slope. A 2D numerical wave tank is considered. At each time step, boundary element method (BEM) is adopted to solve the boundary value problem (BVP) for the velocity potential  $\phi$

and its time derivative  $\phi_t$ . Panels are distributed along the body surface  $S_b$ , free surface  $S_f$ , left vertical wall  $S_{w1}$ , right vertical wall  $S_{w2}$  and the bottom  $S_0$ . (See fig.1). The variation of  $\phi$  (or  $\phi_t$ ) and  $\phi_n$  (or  $\phi_{tn}$ ) over each element are assumed linearly varying. The continuity condition of  $\phi$  (or  $\phi_t$ ) at the intersection points of different boundaries is enforced. Fourth order Runge-Kutta method is applied to update  $\phi$  and  $\eta$  on the free surface for the next time step.

The formulation of body boundary condition for  $\phi$  follows that of Ogilvie (1983), while the body boundary condition for  $\phi_t$  is based on Wu (1998).

Similar to Clement (1996), a coupling of a numerical beach and a piston-like absorbing boundary condition (PABC) is applied to absorb outgoing waves. The numerical beach used here is the same as that of Greco (2001). In order to obtain PABC at each time step, we solve the problem like solving the response of a floating body in waves.

## 3. Verification of the method

The accuracy of the numerical method is an important issue. With the purpose of verification, the free oscillations in a tank, the forced oscillations (wavemaker problem) in a tank, and the nonlinear diffraction and forced oscillations of a horizontal cylinder are studied. The results are presented up to 2<sup>nd</sup> order and comparisons between the analytical results and the experimental results are made.

Cointe et al. (1988) studied two cases of nonlinear transient wave in a rectangular tank. One case is the free oscillation problem, in which an initial displacement is given to the free surface. In the simulation, we set  $L=1.0\text{m}$ ,  $h=0.2\text{m}$  and  $z_0=0.02\cos(\pi x)$  m, where  $L$  is the length of the tank,  $h$  is the water depth and  $z_0$  is the initial displacement of free surface. Fig.2a and Fig.2b show the numerical results of 1<sup>st</sup> order and 2<sup>nd</sup> order wave elevation at

$x=L/8$ , together with the corresponding analytical results at that point. The agreement between the numerical results and the analytical solution is very encouraging.

The forced oscillation (wave maker problem) is also investigated by Cointe et al. (1988). The motion is prescribed on  $S_{w1}$ , while  $S_{w2}$  is fixed. As a useful test to check the accuracy of numerical results, Cointe et al. (1988) suggest the following equation to control the mass conservation at second-order:

$$\int_0^L \eta^{(2)}(x,t) dx = l_1(t) \eta^{(1)}(0,t)$$

Here,  $L$  is the length of the tank;  $\eta^{(1)}(0,t)$  is the 1<sup>st</sup> order wave elevation on  $S_{w1}$ ;  $\eta^{(2)}(x,t)$  is the 2<sup>nd</sup> order wave elevation;  $l_1(t) = F(t) \cdot A_{w1} \sin(\omega t)$  is the displacement of  $S_{w1}$ .  $F(t)$  is the ramp function applied over the first 2 wave periods.  $A_{w1}$  is the amplitude of the displacement of  $S_{w1}$ .

Mass conservation is checked at the end of the third period. The results for first-order and second-order mass check are presented in Fig.3, showing the convergence with the increasing number of elements on the free surface.

Another good way of generating waves in a numerical tank is to feed a theoretical particle velocity profile along the vertical input boundary. This method has been adopted by Koo and Kim (2004) in their fully nonlinear wave tanks, and by Skourup (1996) in 2<sup>nd</sup> order wave tank. However, we must be careful with the mass transport due to the boundary condition that is used. One would expect the increase of mass all the time without a damping zone mechanism that can take mass out of the system. The rate of mass transport is given by Dean and Dalrymple (1991) as  $\rho g a^2 k / (2\sigma)$ , where  $\rho$  is the density of water,  $\sigma$  is the wave frequency,  $g$  is gravity acceleration and  $k$  is the wave number. As shown in Fig.4, the rate of mass transport observed in the 2<sup>nd</sup> order simulation agrees well with that given by Dean and Dalrymple (1991). Note that the mass transport shown in the figure is non-dimensional, and the offset of the two curves is due to the ramp function which is used to give a smooth start of the flow.

Isaacson and Ng (1993) studied the forced oscillation of a horizontal cylinder in deep water. Their method was based on a constant element

method with implicit 2<sup>nd</sup> order Adam-Moulton for the time integration of free surface conditions.  $\phi_t$  was derived by a finite difference method.

In the literature, there are two ways to get the 2<sup>nd</sup> order force acting on the body, i.e. the direct integration method (Pinkster and Oortmerssen, 1977) and indirect method based on Green's 2<sup>nd</sup> identity (Faltinsen, 1976). In the former method, one has to solve the 2<sup>nd</sup> order problem and integrate the pressure on the body surface. In the latter method, instead of solving the 2<sup>nd</sup> order problem, we can obtain the 2<sup>nd</sup> order force by using the boundary conditions and solutions of the linear problems. In this paper, we introduce two artificial velocity potentials and use the indirect method as a tool to check the 2<sup>nd</sup> order numerical results.

The artificial velocity potentials  $\psi_i$  ( $i=1, 2$ ) introduced are similar to Wu and Taylor (2003).  $\psi_i$  satisfies the Laplace equation,  $\psi_i = 0$  on  $z=0$ ,  $\frac{\partial \psi_i}{\partial n} = n_i$  on the mean

position of body surface,  $\psi_i \rightarrow 0$  as  $z \rightarrow -\infty$  and the radiation condition. Green's 2<sup>nd</sup> identity leads to

$$\int_{S_b + S_f + S_\infty} \left( \phi_t^{(2)} \frac{\partial \psi_i}{\partial n} - \psi_i \frac{\partial \phi_t^{(2)}}{\partial n} \right) ds = 0,$$

where  $\phi_t^{(2)}$  is the time derivative of the second order velocity potential  $\phi^{(2)}$ ,  $n$  is the unit normal to the surface, defined as positive pointing out of the fluid domain.

Using the condition for  $\phi_t^{(2)}$  at infinity and the free surface condition for  $\psi_i$ , we obtain

$$\begin{aligned} F_i^{(2)} &= -\rho \int_{S_b} \phi_t^{(2)} n_i ds = -\rho \int_{S_b} \phi_t^{(2)} \frac{\partial \psi_i}{\partial n} ds \\ &= -\rho \left[ \int_{S_b} \psi_i \frac{\partial \phi_t^{(2)}}{\partial n} ds - \int_{S_f} \phi_t^{(2)} \frac{\partial \psi_i}{\partial n} ds \right] \\ &= -\rho \left[ \int_{S_b} \psi_i \frac{\partial \phi_t^{(2)}}{\partial n} ds - \int_{S_f} \phi_t^{(2)} \frac{\partial \psi_i}{\partial n} ds + \int_{S_f} \psi_i \frac{\partial \phi_t^{(2)}}{\partial n} ds \right] \\ &= -\rho \left[ \int_{S_b} \psi_i \frac{\partial \phi_t^{(2)}}{\partial n} ds - \int_{S_f} \phi_t^{(2)} \frac{\partial \psi_i}{\partial z} ds \right] \end{aligned}$$

The analytical solution for  $\psi_i$  for a half circular cylinder has been derived, by taking an image of the body and solving the integral equation directly. The expression for  $\frac{\partial \psi_i}{\partial z}$  on the free surface is found to be

$$\frac{\partial \psi_i}{\partial z} = \begin{cases} -\frac{2}{\pi} \left[ \frac{1}{\gamma} + \frac{1}{2} \ln \left| \frac{\gamma-1}{\gamma+1} \right| \left( 1 + \frac{1}{\gamma^2} \right) \right]; & i=1 \\ -\frac{1}{\gamma^2}; & i=2 \end{cases}$$

Here  $\gamma = \frac{x}{R}$ ,  $R$  is the radius of the cylinder,  $x$  is defined as in Fig.5.

We note that  $\frac{\partial \psi_1}{\partial z}$  has a logarithmic singularity at the intersection point of the free surface and the body. Consequently,  $\psi_1$  is also singular at the intersection point. However, the singularity itself doesn't matter because the integration of the singularity over an infinitesimal area is finite. Fig.6a shows the amplitude of 2<sup>nd</sup> order oscillatory forces due to 2<sup>nd</sup> order velocity potential  $\phi^{(2)}$ . The agreement between the results of direct integration method and that of the indirect method is excellent, indicating the accuracy of the 2<sup>nd</sup> order solution. In Fig.6b and Fig.6c, we also present the amplitude of the total oscillatory forces compared with the theoretical and experimental results of Yamashita (1977) and Kyojuka (1982).

The fixed horizontal cylinder in deep water waves was studied by Isaacson and Cheung (1991). The wave field is separated to the known incident wave  $\phi_i$  and the unknown scattered wave  $\phi_d$ . The second order solution is again checked by Green's 2<sup>nd</sup> identity. (See Fig.7a). The agreement between the direct method and method based on Green's 2<sup>nd</sup> identity is fairly good. The total 2<sup>nd</sup> order horizontal force compared with theoretical and experimental results by Kyojuka (1982) is shown in Fig.7b, while the horizontal mean drift force is checked by Maruo's formula, as shown in Fig.7c.

#### 4. Future work

Verification work is being carried out for 3<sup>rd</sup> order results in two-dimensional problems. This will be done first by considering a 3<sup>rd</sup> order problem where an analytical solution is possible by imposing an artificial body boundary condition. Details will be presented at the conference. In order to investigate ship springing, the method will be generalized to three dimensions with the effect of forward speed.

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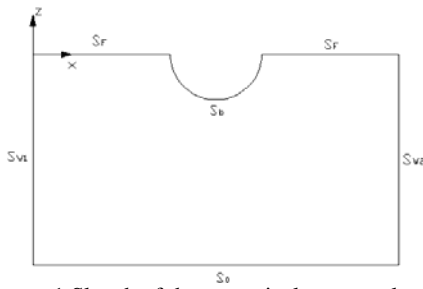


Fig.1 Sketch of the numerical wave tank with the presence of a body.

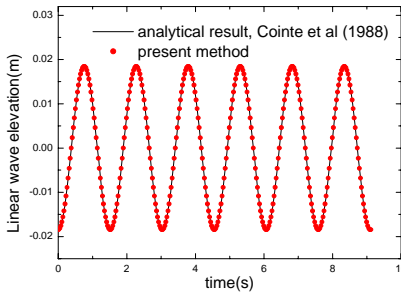


Fig.2a 1<sup>st</sup> order component of wave elevation at  $x=L/8$

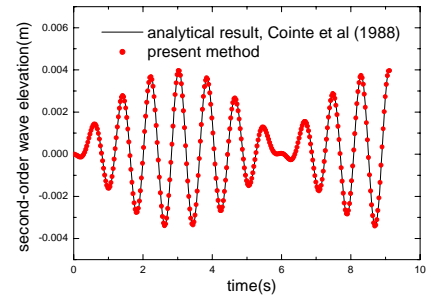


Fig.2b 2<sup>nd</sup> order component of wave elevation at  $x=L/8$

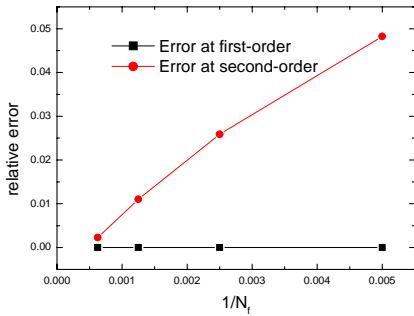


Fig.3 The relative error of the mass conservation.

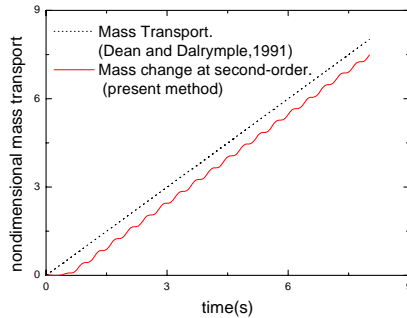


Fig.4 The mass transport into the tank

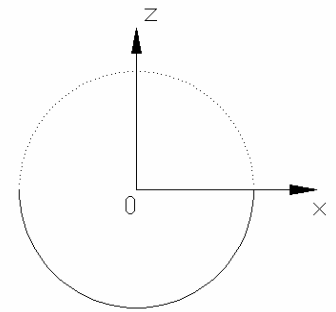


Fig.5 Definition of the coordinate for the problem of  $\psi_i$

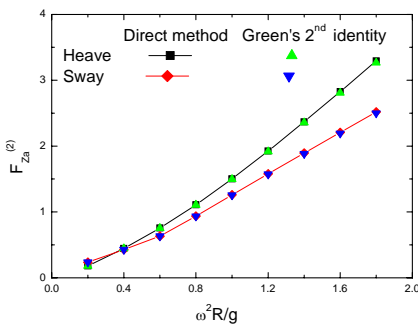


Fig.6a Amplitude of 2<sup>nd</sup> order oscillatory force due to  $\phi^{(2)}$  in the forced oscillation of the cylinder.

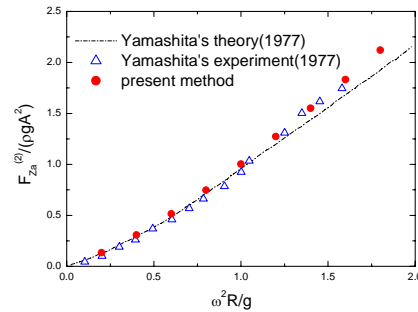


Fig.6b Amplitude of  $F_z^{(2)}$  force in heave motion.  $F_z^{(2)}$  is the total 2<sup>nd</sup> order oscillatory force in vertical direction.

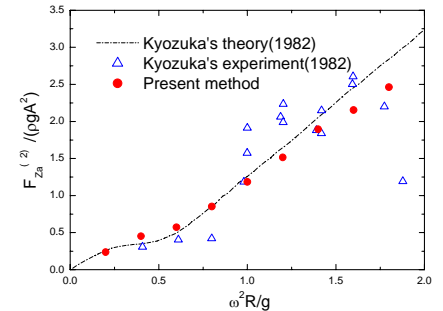


Fig.6c Amplitude of  $F_z^{(2)}$  force in sway.  $F_z^{(2)}$  is the total 2<sup>nd</sup> order oscillatory force in vertical direction.

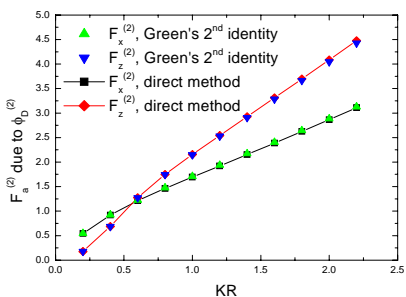


Fig.7a Amplitude of 2<sup>nd</sup> order oscillatory force due to  $\phi_d^{(2)}$  when the cylinder is fixed with incident wave.

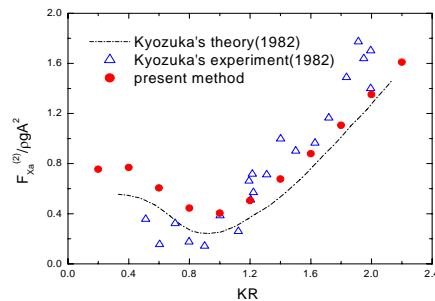


Fig.7b Amplitude of  $F_x^{(2)}$  on a fixed horizontal cylinder with incident wave.

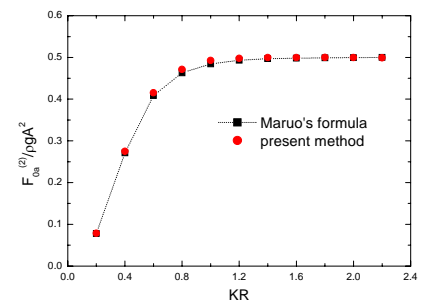


Fig.7c Horizontal mean drift force on a fixed horizontal cylinder with incident wave.